Application of SCS-Curve Number Method to estimate Runoff using GIS for Gali-Bandawa Watershed

Zahraa Mahmood Klari¹ Sayran Ahmed Ibrahim²

¹,²Water Resources Department, College of Engineering, University of Duhok, Kurdistan Region – IRAQ

ABSTRACT

In any hydrologic study, the most important parameter is the runoff which is necessary for designing any hydraulic structure, and for determining the risk of flood. As there is a scare in the availability of runoff data in many sites, hydrologists have developed indirect methods to determine the runoff to accelerate the program of watershed management for conserving and developing water resources management. Many methods are used to estimate the runoff; Soil conservation curve number (SCS-CN) method is widely used and gives a reliable result compared with other methods. The present study aims to calculate the surface runoff depth depending on the SCS-CN method using a Geographic information system (GIS). For this Gali-Bandawa watershed in Duhok, north of Iraq has been selected, the geographical area of this watershed is about 92Km² and the average annual rainfall is around 620mm, the weighted CN is 76. The results show that the depth of annual average runoff for the Gali-Bandawa watershed is 70mm, and the average volume of runoff from the same watershed is 6470360 m³. The amount of runoff represents 11.4% of the total annual rainfall. This approach could be applied in other Iraqi's watersheds for the planning of various conservation measures.

Keywords: GIS, SCS-CN method, rainfall-runoff, FORTRAN programming, Gali-Bandawa watershed.

1. Introduction

Estimation of runoff is an important part of this study because Iraq is within the semi-arid region. The climate of the region is characterized by its fluctuation in rainfall and periods of droughts (Al-Ansari, 1998). Recently, the water scarcity issue is becoming more serious due to several factors. Among them is the increasing water demand, high population rate, effect of global warming and poor management and planning of the water resources during the last four decades. In addition, water policies of the neighboring countries enforced another burden where huge dams were built on the upper parts of the Tigris and Euphrates Rivers in Syria and Turkey. And this led to the reduction of the flow rate of both rivers inside of Iraq. So it is important to build dams in suitable areas to guarantee water reserving for drought seasons (Al-Ansari, 2011). To meet the scarcity of water shortage, building many dams that depend on rain water as a major source is a vital serious matter that must be studied by hydrologists and administrators, especially in areas where there is no measurement of the runoff. Only rainfall measurements exist, so several methods are used to estimate runoff. Most hydrological studies faced a problem with determining the amount of runoff, especially when there are records of precipitation and no records for runoff. Runoff is happening when the precipitation speed exceeds the speed of infiltrated water into the soil. All area that covers all lands contributing runoff to a common point called a watershed (Al-Jabari, Abu-Shark, and Al-Mimi, 2009). In Iraq, the accurate information of runoff is not available in most areas and is limited. The Soil Conservation Services Hydrologists constantly face the problem of estimating direct runoff in which no available records for a specific watershed. Soil Conservation Service (USDA, 1985) SCS curve number method is an acceptable tool in hydrology, which uses a factor for land conditions called "the curve number" and its responsiveness to four important catchment properties, i.e. land use, soil type, antecedent moisture condition, and surface condition. In a recent study of runoff (Lange, Ghanem, Hussary, Leibundgu, and Greenbaum, 2000) have studied the runoff on a
steep 180 m² Mediterranean Karts environment. The results showed that on a dry plot about 16 mm of rainfall was needed before terrain other bar rock generated runoff. Overall 16% of rainfall turns into runoff, while in the following day 73% of the applied rainfall arrived at the outlet of the wet plot. Shaded and Almasri (2010), have assessed the flow modeling in West Bank watersheds using GIS and SCS – CN method. They demonstrated that by integrating of GIS with SCS – CN method, a powerful tool will be provided to estimate the runoff. Another study has been studied in the Kurdistan Region of Iraq drought period during the seasons 2007-2008 and 2008-2009 that affected the human and economic activities of the region. Macro rain-water harvesting (Macro RWH). This technique is based on the Soil SCS-CN method and WMS to estimate the runoff. The results of the application of the WMS model showed that about 10.76 million cubic meters could be harvested. The results also showed that the quantity of the harvested runoff was highly affected by rainfall depth, curve number values, antecedent moisture conditions (AMC), and the area of the basins (Zakaria, Mustafa, Mohammed, Ali, Al-Ansari, and Knutsson, 2013). Shah, Motiani, Prakash, and Mehmood (2017) have determined the runoff depth using the Soil conservation Service (SCS-CN) method using a Geographical Information System (GIS) environment. For this, Meshwo River Watershed located in Dhansura - Aravalli district of Gujarat was selected and the Natural Conservation Service- CN method was adopted for estimating the runoff. Average rainfall data was used to calculate the average runoff. The runoff amount represented about 26%of the average rainfall for the year 2015. Khidir and AL-Sha’ar (2017) applied Watershed Modeling System (WMS) to estimate the runoff using three different synthetic unit hydrograph methods, were NRCS, Snyder, and Clark, and to choose the best one according to some statistical test result, he found that the Snyder method results is are closer to the recorded data. Eshanthini Vijayalakshmi, and Raji (2018) used GIS and remote sensing as a tool to calculate the volume of runoff applying SCS-CN mode, they estimate CN values depending on the natural resources conservation standard table. They concluded that the model SCS along with GIS would give a more reliable runoff for ungagged watersheds

In this paper, Gali-Bandawa was selected as a study area because it has been suggested by directorate of Duhok dam for constructing a dam at its outlet. To estimate runoff, GIS tool was used to prepare, classify and identify both soil type and land use maps for the selected area, and by develop the runoff curve number map, Finally, SCS curve number method used to estimate runoff with help of a FORTRAN program to facilitate the solution to obtain the runoff from daily rainfall data.

2. Study Area

The study area, named Gali Bandawa or Bandawa, is located in Duhok city (North of Iraq). The watershed outlet coordinate has a Latitude (N) is 36º46'28" and a Longitude (E) is 43º04'30" Figure (1). The watershed has a geographical area around 92Km²; the Basin slope is 18.2% and the maximum slope length is 16.1km. The area, in general, has a moderate slope around the dam and a steep slope in some higher parts of the basin, especially in the mountains. The maximum, mean, and minimum elevations are correspondingly 1330m, 502m found by GIS tool. The area occupies a valley of seasonal streamflow, and the source of runoff is the rainfall.

![Figure 1: Location of Gali-Bandawa Watershed](image-url)
3. Materials and Method

In this study, a variety of data including the Digital Elevation Model (DEM 30m), Metrological data of rainfall have been obtained for the year 2001-2014 from the Directorate of Duhok Dam in Iraq. Land use map is obtained by classifying the Landsat image from the USGS website. In this study, the runoff estimation is summarized in a flowchart as shown in Figure (2).

![Flowchart of Rainfall-Runoff Estimation processes.](image)

3.1 Soil Type Map Preparation

The Soil Map of a part of Duhok governorate by Ameen (2016). It’s extracted to fit the selected Gali-Bandawa Watershed using GIS tools as shown in Figure (4).

![Soil Type Map of Gali Bandawa.](image)

Two types of soil are classified for Gali-Bandawa watershed; they are Type B and Type C, both types of soil are described as shown in Table (1).

<table>
<thead>
<tr>
<th>Type of Soil</th>
<th>Drainage Area = 92 sq. Km</th>
<th>Percentage</th>
</tr>
</thead>
<tbody>
<tr>
<td>B</td>
<td>Moderately deep, well-drained, with moderately fine to coarse texture. Soils having a moderate infiltration rate. Clay loam, shallow sandy loam.</td>
<td>40 %</td>
</tr>
<tr>
<td>C</td>
<td>These soils have a slow rate of water transmission.</td>
<td>60 %</td>
</tr>
</tbody>
</table>

4.2 Land Use and Land Cover preparation

The Landsat image is used to create land use/Landcover map Figure (3) using Arc GIS to prepare a land-use pattern of the study area.

![Land use map of Gali Bandawa.](image)

The original land-use types were grouped into six groups as cultivated, forest, tree, and impervious, urban, and bare land. Table (2) shows the percentage of land use classes after classification is satisfied.

<table>
<thead>
<tr>
<th>NO.</th>
<th>Land Use Area=92 sq.km</th>
<th>Percentage %</th>
</tr>
</thead>
<tbody>
<tr>
<td>1</td>
<td>Frost</td>
<td>7</td>
</tr>
<tr>
<td>2</td>
<td>Developed land</td>
<td>11</td>
</tr>
<tr>
<td>3</td>
<td>Impervious land</td>
<td>8</td>
</tr>
<tr>
<td>4</td>
<td>Tree</td>
<td>58</td>
</tr>
<tr>
<td>5</td>
<td>Bare land</td>
<td>7</td>
</tr>
</tbody>
</table>
4.3 SCS CURVE NUMBER METHOD
The SCS-CN method is developed by the USDA in 1954. This method is based on a water balance equation and other two functional hypotheses. The first hypothesis shows that both quantity are the same, depth of runoff \( q \) is less than or equal to depth of precipitation \( P \), also \( F_a \) is the water depth retained in a watershed is equal or less than to some maximum potential retention \( S \). \( I_a \) is initial abstraction for which no runoff occur, so \( P-I_a \) is the potential runoff (Chow, 2002). As shown in equation (1)

\[
F_a = \frac{q}{P-I_a}
\]

While the other hypothesis assumes that the amount of initial abstraction is part of the maximum potential retention (Engineering Hydrology Handbook by Subramanya K).

The weighted curve number is calculated after recognizing the CN for N number of land classes using equation (2).

\[
CN_w = \frac{\sum_{i}^{N} CN_i \cdot A_i}{\sum_{i}^{N} A_i}
\]

Where: \( CN_w \) is the weighted curve number, \( CN_i \) is the curve number for a particular land class unit i to n, and \( A_i \) is the area of the particular land class i.

\[
q = \begin{cases} 
(P-I_a)^2 / (P-I_a+S) & \text{if } P > 0.05 \, S \\
0 & \text{if } P < 0.05 \, S 
\end{cases}
\]

Where: \( q \) is the runoff depth (L), \( P \) is the total depth of rainfall (L), and \( I_a \) is the initial abstraction (L). Finally, \( S \) is the abstraction coefficient, it represents the upper infiltration in soil and is increased with decreasing of curve number value.

Recently, It has been assumed that \( I_a = 0.2 \, S \) (Chow, 2002), another recent research estimated that \( I_a = 0.05 \, S \) may by more accurate relationship (Hawkins et al., 2002). The relationship \( I_a = 0.2 \, S \) has been used widely for the experimental study of small watersheds; more recent methods were analyzed to determine the relationship between \( I_a \) and \( S \) with many other data of rainfall and runoff from numerous U.S. watersheds done by Hawkins et al. (2002). The runoff depth was improved to the following equation:

\[
q = \frac{(P-0.05 \, S_{0.05})^2}{P+0.95 \, S_{0.05}}
\]

In equation (6) the term \( S_{0.05} \) is different from the one that used for runoff estimation with \( I_a = 0.2 \, S \) (here \( S \) represented by \( S_{0.2} \)). A relationship was derived \( S_{0.05} \) from \( S_{0.2} \) as shown in the following equation:

\[
S_{0.05} = 1.33 \, S_{0.2}^{1.15}
\]

4.4 Antecedent Moisture Condition, AMC
Antecedent moisture condition (AMC) of the rainfall-runoff event refers to the amount of moisture content found in the soil at the beginning before the rainfall. Both initial abstraction and infiltration are governed by AMC.

If the summation of rainfall for the previous five days is less than 1.4 inches; dry condition of soil is assumed, and CN is converted to (CNI) using equation (8)

\[
CN(I) = \frac{4.2 \, CN(II)}{10^{-0.059 \, CN(II)}}
\]

But, if the summation of rainfall is greater than 2.1 inches; then wet condition of soil is assumed, and CN is converted to (CNIII) using equation (9)

S = \frac{25400}{CN} - 254

\[ \text{CN(III)} = \left( \frac{23\text{CN(II)}}{10 + 0.13\text{CN(II)}} \right) \]  

(9)

The antecedent 5 days rainfall has been used as an index to fit the antecedent moisture conditions (Al-Jabari et. al., 2009) in Table (3)

<table>
<thead>
<tr>
<th>AMC</th>
<th>Total 5-day antecedent rainfall (mm)</th>
<th>Dormant season</th>
<th>Growing season</th>
</tr>
</thead>
<tbody>
<tr>
<td>I</td>
<td>&lt;12.7</td>
<td>&lt; 35.6</td>
<td></td>
</tr>
<tr>
<td>II</td>
<td>12.7 – 27.9</td>
<td>35.6 – 53.3</td>
<td></td>
</tr>
<tr>
<td>III</td>
<td>&gt; 27.9</td>
<td>&gt; 53.3</td>
<td></td>
</tr>
</tbody>
</table>

4.5 FORTRAN (90) Programming

Language of FORTRAN programming was the highest level language, H.L.L. developed in 1954 for IBM Company. It stands for FORMula-TRANslation, used for solving both scientific and engineering issues by using symbolic names to inform mathematical equations in a comprehensible form. FORTRAN program also control structures than include conditional IF statement, DO loops (repeated loops), and a GOTO statement (Hemmendinger, 2000).

5. Results and Discussions

By using the extension of Hec-GeoHMS in GIS, the curve number Grid map can be created by merging land use and soil type maps as shown in figure (4)

The weighted curve number for the Gali Bandawa watershed is:

\[ \text{CNw} = \frac{7007}{92} = 76 \]

The CN is rounded 76 as the normal condition (AMCII), CN for the other two conditions; the dry condition (AMCI) and the wet condition (AMCIII) were obtained using equations (6) and (7):

\[ \text{CN (I)} = 58, \]
\[ \text{CN(III)} = 88 \]

Before estimation of the runoff depth q in equation (2) the value of (S) must be determined for each antecedent moisture condition (AMC) as summarized in the Table (5).

<table>
<thead>
<tr>
<th>AMC condition</th>
<th>(CN)</th>
<th>S</th>
<th>P&gt;0.05</th>
</tr>
</thead>
<tbody>
<tr>
<td>I</td>
<td>58</td>
<td>7.241</td>
<td>2.172</td>
</tr>
<tr>
<td>II</td>
<td>76</td>
<td>3.158</td>
<td>0.947</td>
</tr>
<tr>
<td>III</td>
<td>88</td>
<td>1.364</td>
<td>0.409</td>
</tr>
</tbody>
</table>
As a result of the calculations, based on the SCS method, it was found that the average annual surface runoff rate (depth) for the period (2001-2014) in Bandawa watershed is around 70 mm which represented 11.4% of the total annual rainfall. The annual rainfall and runoff during (2001-2014) in the study area are shown in Table (6).

<table>
<thead>
<tr>
<th>Year</th>
<th>Annual Rainfall (mm)</th>
<th>Annual Runoff (mm)</th>
</tr>
</thead>
<tbody>
<tr>
<td>2001</td>
<td>445.10</td>
<td>70.02</td>
</tr>
<tr>
<td>2002</td>
<td>742.28</td>
<td>178.98</td>
</tr>
<tr>
<td>2003</td>
<td>689.46</td>
<td>116.10</td>
</tr>
<tr>
<td>2004</td>
<td>1,068.61</td>
<td>73.49</td>
</tr>
<tr>
<td>2005</td>
<td>511.65</td>
<td>34.83</td>
</tr>
<tr>
<td>2006</td>
<td>806.32</td>
<td>111.26</td>
</tr>
<tr>
<td>2007</td>
<td>430.46</td>
<td>47.02</td>
</tr>
<tr>
<td>2008</td>
<td>387.85</td>
<td>10.46</td>
</tr>
<tr>
<td>2009</td>
<td>587.26</td>
<td>57.93</td>
</tr>
<tr>
<td>2010</td>
<td>363.10</td>
<td>11.64</td>
</tr>
<tr>
<td>2011</td>
<td>464.73</td>
<td>17.93</td>
</tr>
<tr>
<td>2012</td>
<td>559.95</td>
<td>27.70</td>
</tr>
<tr>
<td>2013</td>
<td>836.55</td>
<td>59.17</td>
</tr>
<tr>
<td>2014</td>
<td>778.50</td>
<td>168.09</td>
</tr>
<tr>
<td>Mean</td>
<td>619.42</td>
<td>70.33</td>
</tr>
</tbody>
</table>

The findings showed that the maximum runoff happened in month January and the minimum value was in month May, and finally, there was no runoff in months June, July, and August as there was no rainfall as shown in figure (5).

![Figure 5: Average rainfall and runoff for the period (2001-2014)](image)

6. Conclusion

SCS-CN method has been used for calculating the runoff for Gali Bandawa ungauged watershed having an area of 92 km². Land use and soil type maps have been prepared using GIS to find out the CN map of the study area to determine the runoff. Types B and type C of soil have been classified within the region, six classes of land use were categorized, namely; forest, urban, impervious land, tree, bare land, and cultivated land. CN values range from 58 to 92, and weighted CN is 76. Daily rainfall data for the years 2001-2014 was used to compute the daily runoff depth at these years. It should be noticed that GIS tool helps in more accurate determination of morphological of the basin and classification of land-use map.

FORTRAN programming has been used to apply the SCS-CN method on the daily rainfall data to facilitate the calculations as shown in appendix A. The result showed that the average annual runoff was 70.33 mm and represent 11.4% of the average annual rainfall. Besides that, the maximum runoff happened in month January, and minimum values of runoff happened in May. The estimated runoff of this area is useful for managing the Gali-Bandawa watershed. Various conservation measures could be planned using the same applied approach.

7. REFERENCES


11. Engineering Hydrology Handbook of Subramanya K


17. UNDP (2011) Drought impact assessment, recovery and mitigation framework and regional project design in Kurdistan region.


Appendix A

Computer programing using FORTRAN 90

Real R(6000),CN(20),AREA(20)
open(5, file='Rain.in')
open(6, file='Rain.out')
! calculating CNII for cni=12 and area=12
  do i=1,12
    read(5,*)CN(i),AREA(i)
  enddo
  do i=1,12
    sumCN_area=sumCN_area+cn(i)*AREA(i)
    sumAREA=sumAREA+AREA(i)
  enddo
  CNII=sumCN_area/sumAREA
  write(6,*)'CNII=',CNII

!---- reading and wrtiting all rainfall data ----
  do i=1,5540
    read(5,*)R(i)
  enddo
  ! write(6,*)R(i)
  endif
  k=k+1
  ! write(6,*)'last value',R(j), sumR
  if (i==n-1) goto 30
  if (i==n-6) goto 30
  enddo
  30 write(6,*)'end of program'
  stop
end

Appendix B

Computer programing using FORTRAN 90

Real R(6000),CN(20),AREA(20)
open(5, file='Rain.in')
open(6, file='Rain.out')
! calculating CNII for cni=12 and area=12
  do i=1,12
    read(5,*)CN(i),AREA(i)
  enddo
  do i=1,12
    sumCN_area=sumCN_area+cn(i)*AREA(i)
    sumAREA=sumAREA+AREA(i)
  enddo
  CNII=sumCN_area/sumAREA
  write(6,*)'CNII=',CNII

!---- reading and wrtiting all rainfall data ----
  do i=1,5540
    read(5,*)R(i)
  enddo
  ! write(6,*)R(i)
  endif
  k=k+1
  ! write(6,*)'last value',R(j)
  ! write(6,*)'sumR=',sumR
  if (sumR<1.4*25.4) then
    !sumR<1.4*25.4 then
    CNI=(4.2*CNII)/(10-0.058*CNII)
    s_2=(25400/CNII)-254
    s_5=1.33*s_2
    q=(R(j+1)-0.05*s_5)**2/(R(j+1)+0.95*s_5)
    write(6,*)'mediam condition','CNII=',CNII ,' q=',q,' rain=',R(j+1)
    !---------------------------------------------------------------------
  elseif(sumR>2.1*25.4) then
    CNII=(23*CNII)/(10+0.13*CNII)
    s_2=(25400/CNII)-254
    s_5=1.33*s_2
    q=(R(j+1)-0.05*s_5)**2/(R(j+1)+0.95*s_5)
    write(6,*)'wet conditions' CNII,'CNII ',' q=',q,' rain=',R(j+1)
    !write(6,*)' s_2,s_2,s_5,s_5,' q,q
    !else if(sumR<2.1*25.4) then
    CNII=CNII
    s_2=s_2
    s_5=s_5
    q=q
    !write(6,*)' dry conditions',CNII,CNII,' q=',q,' rain=',R(j+1)
    !write(6,*)' s_2,s_2,s_5,s_5,' q,q
    !---------------------------------------------------------------------
  elseif(sumR>=35.56.and.sumR<=53.34) then
    !sumR>=35.56 and sumR<=53.34 then
    s_2=(25400/CNII)/254
    s_5=1.33*s_2
    q=(R(j+1)-0.05*s_5)**2/(R(j+1)+0.95*s_5)
    !write(6,*)'very wet conditions',CNII,CNII,' q=',q,' rain=',R(j+1)
    !write(6,*)' s_2,s_2,s_5,s_5,' q,q
    !---------------------------------------------------------------------
  else
    endif
  k=k+1
  !write(6,*)'last value',R(j)
  !write(6,*)'sumR=',sumR
  if (i==n-1) goto 30
  if (i==n-6) goto 30
  enddo
  30 write(6,*)'end of program'
  stop
end